

Présentations des membres du CFMR au XIIème Congrès
International de Mécanique des Roches

Post-closure hydromechanical behaviour of a backfilled cavity

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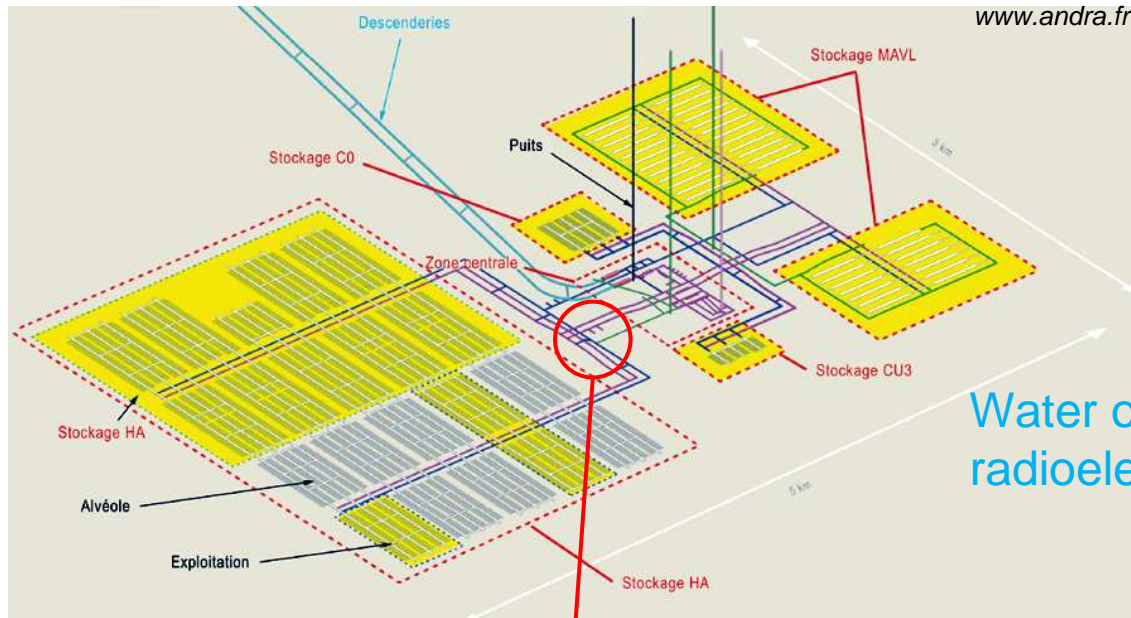
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Plan

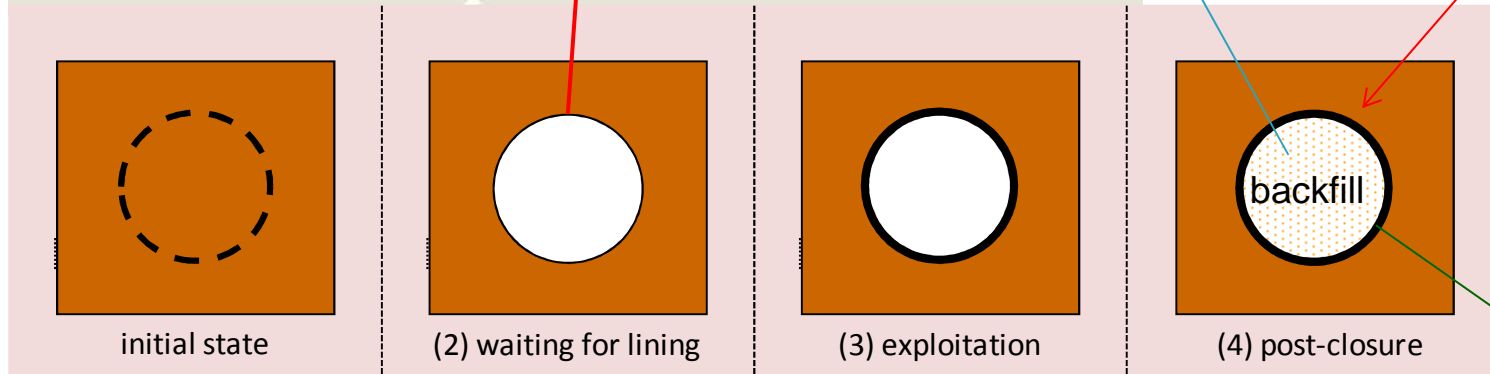
- ▶ Problem setting
- ▶ Linear behaviour – Analytical approach
 - poroelasticity
 - poroviscoelasticity
- ▶ Non linear behaviour – Numerical approach
 - Poro-viscoplasticity + isotropic damage
 - poro-viscoplasticity + anisotropic damage
- ▶ Conclusions

Idealized problem



Water out-flow +
radioelements (?)

Ground convergence
(creep, damage, HM
diffusion..)



Liner degradation:
Leaching or
Brutal failure ?

$t_0=0$

t_1
(1) excavation

$t_1 = 1$ day

t_2
support installation

$t_2 = 2$ months

t_3
backfill

$t_3 = 150$ years

Methodology

1. Linear material behaviour :

Poroelasticity

poro-viscoelasticity

Analytical ou Quasi-analytical solutions

(Benchmarks)



2. Non-linear behaviors

Poro-viscoplasticity,

Iso/aniso damage, ...

Numerical solutions

Wong, H., Morvan, M., Deleruyelle, F. & Leo, C.J. 2008a Analytical study of mine closure behaviour in a poro-elastic medium, *Computers and Geotechnics* 35(5):645-654.

Wong, H., Morvan, M., Deleruyelle, F. & Leo, C.J. 2008b Analytical study of mine closure behaviour in a poro-visco-elastic medium, *International Journal for Numerical and Analytical Methods in Geomechanics* 32(14):1737-1761.

Dufour, N., Leo, C.J., Deleruyelle, F. & Wong, H. 2009 Hydromechanical responses of a decommissioned backfilled tunnel drilled into a poro-viscoelastic medium, *Soils and Foundations* 49(4):495-507.

Dufour, N., Wong, H., Deleruyelle, F. & Leo, C.J 2010 *Hydromechanical post-closure behaviour of a deep tunnel taking into account a simplified life cycle*, *Int. J of Geomechanics*, in press.

+ ...

1. Linear material behaviour :
Poroelasticity
Poro-viscoelasticity
Analytical or quasi-analytical
solutions

Principal equations

Poroelasticity

- Behaviour laws (*Coussy, 2004*)

$$\begin{aligned}\boldsymbol{\sigma} - \boldsymbol{\sigma}^0 &= \left(K - \frac{2G}{3}\right) \boldsymbol{\epsilon}(\mathbf{u}) \mathbf{1} + 2G\boldsymbol{\epsilon}(\mathbf{u}) - b(p - p_0) \mathbf{1} \\ \phi - \phi^0 &= b\boldsymbol{\epsilon}(\mathbf{u}) + \frac{p - p_0}{N}\end{aligned}$$

- Darcy's law $\mathbf{V}^D = \lambda_h(-\mathbf{grad}(p) + \rho_f \mathbf{g})$

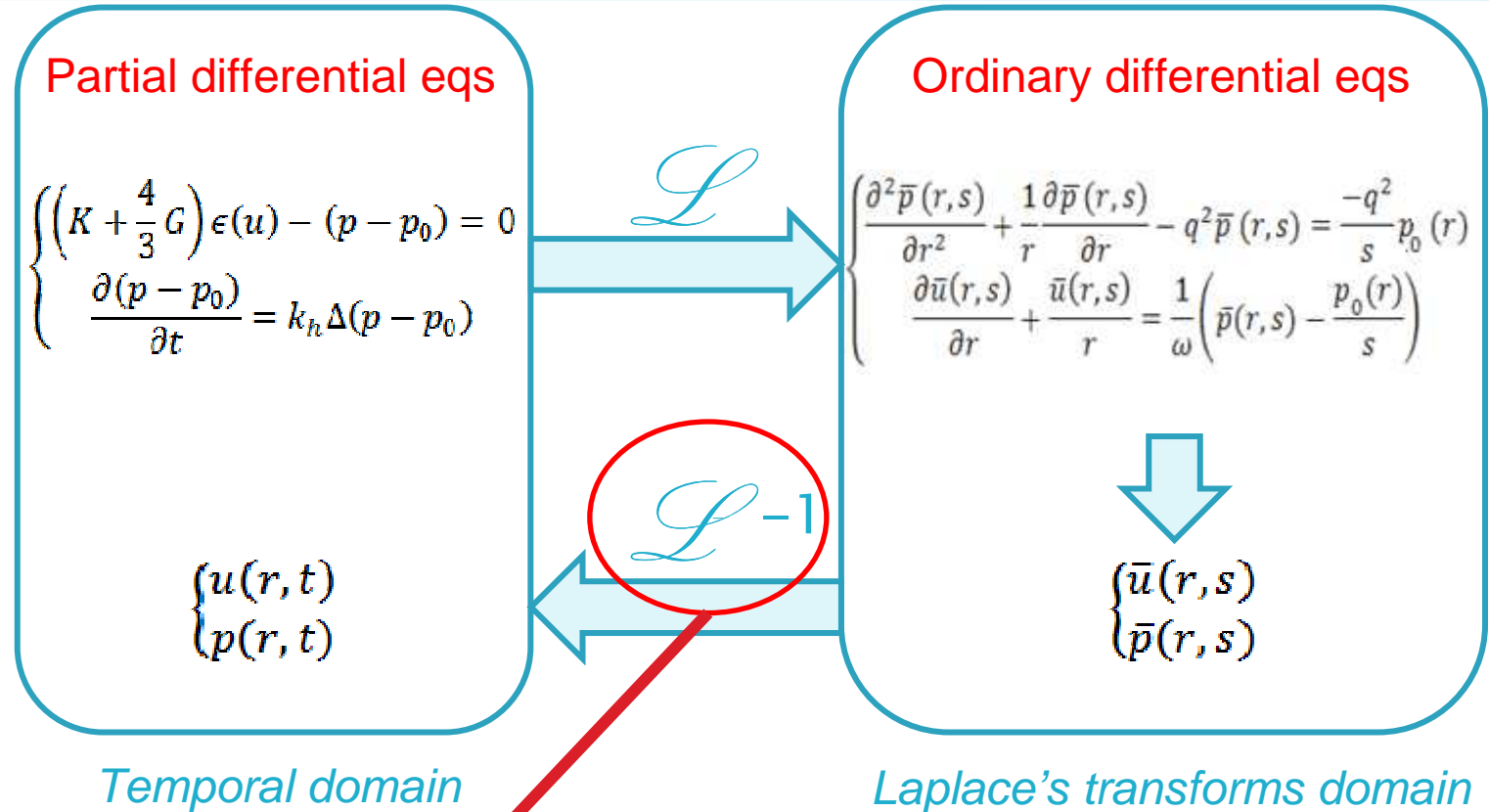
- Equilibrium $\mathit{div}(\boldsymbol{\sigma} - \boldsymbol{\sigma}^0) = 0$

2-fields problem (u, p) : cylindrical or spherical symmetry

$$\left\{ \begin{array}{ll} \left(K + \frac{4}{3}G\right) \boldsymbol{\epsilon}(\mathbf{u}) - (p - p_0) = 0 & \boldsymbol{\epsilon}(\mathbf{u}) = \frac{\partial u}{\partial r} + \frac{u}{r} \quad \text{Circular tunnels} \\ \frac{\partial(p - p_0)}{\partial t} = k_h \Delta(p - p_0) & \boldsymbol{\epsilon}(\mathbf{u}) = \frac{\partial u}{\partial r} + \frac{2u}{r} \quad \text{Spherical cavities} \end{array} \right.$$

Analytical solution : tools

Laplace Transform



Stehfest's (1970)

$$f(t) \cong \frac{\ln 2}{t} \sum_{n=1}^N C_n \bar{f} \left(n \frac{\ln 2}{t} \right) \quad C_n = (-1)^{n+\frac{N}{2}} \sum_{k=\text{Int}\left(\frac{n+1}{2}\right)}^{\min\left(n, \frac{N}{2}\right)} \frac{k^{\frac{N}{2}} (2k)!}{\left(\frac{N}{2} - k\right)! k! (k-1)! (n-k)! (2k-n)!}$$

Analytical solution

Poroelastic rock, spherical cavities

$$u'(r', t') = \frac{(p'_0 - 1)}{\omega r'^2} \left[g(r', t', \Omega_1) - g(r', t', \Omega_2) - A\varphi(r', t', \Omega_1) \right. \\ \left. - B\varphi(r', t', \Omega_2) - C\varphi(r', t', -i\sqrt{\kappa'}) - D\varphi(r', t', i\sqrt{\kappa'}) \right]$$

$$p'(r', t') - p'_0 = \frac{1 - p'_0}{r'} \kappa' \left\{ A^* \varphi(r', t', \Omega_1) + B^* \varphi(r', t', \Omega_2) + C^* \varphi(r', t', -i\sqrt{\kappa'}) + D^* \varphi(r', t', i\sqrt{\kappa'}) \right\}$$

with

$$g(r', t', h) = \frac{1}{\Omega_1 - \Omega_2} \left\{ \left(\frac{1}{h} + r' \right) \exp(-h(r'-1) + h^2 t') \operatorname{erfc} \left(\frac{r'-1}{2\sqrt{t'}} - h\sqrt{t'} \right) - \frac{1}{h} \operatorname{erfc} \left(\frac{r'-1}{2\sqrt{t'}} \right) \right\}$$

$$\varphi(r', t', h) = L^{-1} \left\{ \frac{e^{-q(r'-1)}}{(q-h)} \right\} \\ = \frac{1}{\sqrt{\pi t'}} \exp \left(-\frac{(r'-1)^2}{4t'} \right) + h \exp(-h(r'-1) + h^2 t') \operatorname{erfc} \left(\frac{r'-1}{2\sqrt{t'}} - h\sqrt{t'} \right)$$

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Quasi-analytical solution phases 1, 2, 3

ⓐ 1st stage : instantaneous excavation

$$p^{(1)}(r) = p_0 \quad u^{(1)}(r) = -\frac{\Sigma_0}{2G} \frac{a^2}{r}$$

ⓑ 2nd et 3rd stages : unlined gallery, pore pressure dissipation

$$p^{(2)}(r, s) = \frac{p_0}{s} \left[1 - \frac{K_0(qr)}{K_0(qa)} \right]$$
$$\overline{u^{(2)}}(r, s) = \frac{p_0}{qs\omega K_0(qa)} \left[K_1(qr) - \frac{aK_1(qa)}{r} \right] - \frac{a^2}{r} \frac{\Sigma_0}{2Gs}$$

Quasi-analytical solution

Stage 4: backfill + post-closure

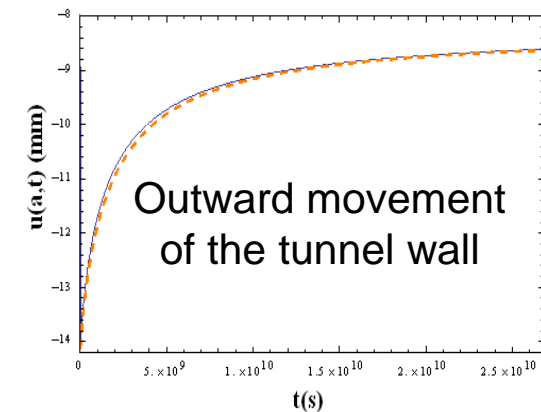
Numerical inversion (Stehfest, 1970) $\rightarrow p^{(4)}(r,t), u^{(4)}(r,t)$

$$\overline{\hat{p}}^{(4)}(r,s) \approx A^{(4)}(s)K_0(qr) + p_0 \sum_{n=1}^N \frac{C_n}{n} \left[\frac{1}{s} - \frac{t_3}{(st_3 - n \ln 2)} \frac{K_0(\alpha_n r)}{K_0(\alpha_n a)} \right]$$

$$\overline{\hat{u}}^{(4)}(r,s) \approx \frac{C^{(4)}(s)}{r} + \frac{r}{2\omega} \left(A^{(4)}(s)K_0(qr) - p_0 t_3 \sum_{n=1}^N \frac{C_n}{n(st_3 - n \ln 2)} \frac{K_0(\alpha_n r)}{K_0(\alpha_n a)} \right)$$

$$A^{(4)}(s) = \frac{p_0}{\omega s K_0(qa) + 2G\sqrt{k_h} s K_1(qa)} \left[\sum_{n=1}^N \frac{C_n}{(st_3 - n \ln 2)} \left(\omega a \ln 2 + 2G \sqrt{\frac{k_h \ln 2 t_3}{n}} \frac{K_1(\alpha_n a)}{K_0(\alpha_n a)} \right) \right]$$

$$C^{(4)}(s) \approx \frac{a^2}{2} \left[\left(\frac{1}{\omega} - \frac{1}{G} \right) \left(p_0 t_3 \sum_{n=1}^N \frac{C_n}{n(st_3 - n \ln 2)} - A^{(4)}(s)K_0(qa) \right) + \frac{1}{Gs} (-\Sigma_0 + p_0) \right]$$



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Analytical solution

Poroviscoelasticity, spherical cavities

Stress-strain:

$$(\sigma - \sigma_0) + (p - p_0) = K \hat{\otimes} \epsilon$$

$$s_{ij} - s_{ij}^0 = 2\mu \hat{\otimes} e_{ij}$$

Stieltjes convolution :

$$x \hat{\otimes} y \equiv \int_{0^-}^t x(t - \tau) \dot{y}(\tau) d\tau$$

Elastic « functions »:

$$K(t) = \left[K_\infty - (K_\infty - K_0) \exp\left(-\frac{t}{\tau_r}\right) \right] H(t)$$

$$\mu(t) = \left[\mu_\infty - (\mu_\infty - \mu_0) \exp\left(-\frac{t}{\theta_r}\right) \right] H(t)$$

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Analytical solution

Poroviscoelasticity, spherical cavities

$$\frac{r'}{\kappa' \omega'_e (1 - p'_0)} p' - p'_0(r', t') = H_1(r', t') \otimes \sum_{k=1}^6 A_k e^{-y_k t'} + H_2(r', t') \otimes \sum_{k=1}^6 B_k e^{-y_k t'}$$

$$H_1(t') = L^{-1} \left[e^{-q'(r'-1)} \right] = e^{-v'_c t'} \left\{ f(t') - \int_0^{t'} \sqrt{\frac{v'_c (v'_r - v'_c) \tau}{t' - \tau}} J_1 \left(2\sqrt{v'_c (v'_r - v'_c) \tau (t' - \tau)} \right) f(\tau) d\tau \right\}$$

$$\text{with } f(t') = \frac{r'-1}{2\sqrt{\pi} t'^3} \exp \left[(2v'_c - v'_r) t' - \frac{(r'-1)^2}{4t'} \right]$$

$$H_2(t') = L^{-1} \left[q' e^{-q'(r'-1)} \right] = e^{-v'_c t'} \left\{ g(t') - \int_0^{t'} \sqrt{\frac{v'_c (v'_r - v'_c) \tau}{t' - \tau}} J_1 \left(2\sqrt{v'_c (v'_r - v'_c) \tau (t' - \tau)} \right) f(\tau) d\tau \right\}$$

$$\text{with } g(t') = \frac{1}{4\sqrt{\pi}} \frac{[(r'-1)^2 - 2t']}{t'^{5/2}} \exp \left[(2v'_c - v'_r) t' - \frac{(r'-1)^2}{4t'} \right]$$

Wong, H., Morvan, M., Deleruyelle, F. & Leo, C.J. 2008a Analytical study of mine closure behaviour in a poro-visco-elastic medium, *International Journal for Numerical and Analytical Methods in Geomechanics* 32(14):1737-1761.

2. Non-linear behaviors
Poro-viscoplasticity,
Iso/anisotropic damage, ...
Numerical solutions

Cast3M : superposition of models

1 FEM mesh
1 displacement field



CAST3M
Huge library of material models

(1) Monophasic model

(2) Poroelastic model

Interface elements
liner - rock
backfill - liner



possible gap opening

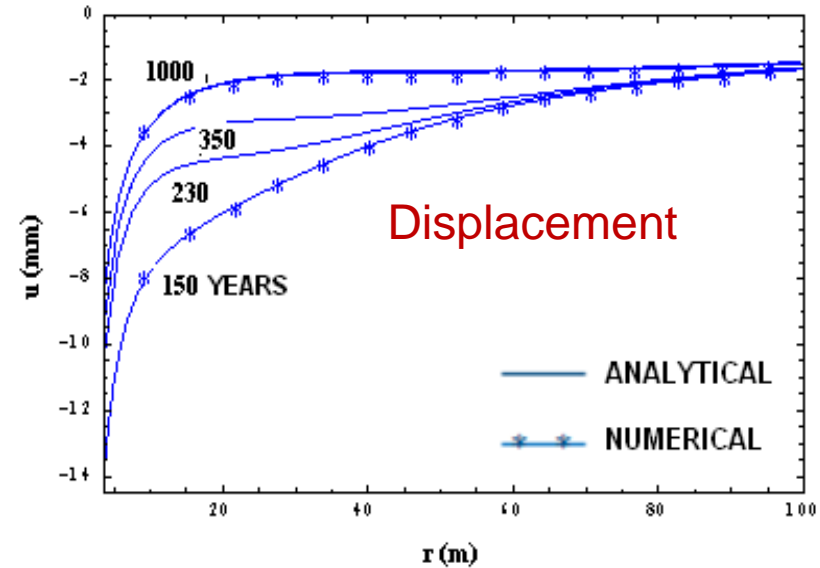
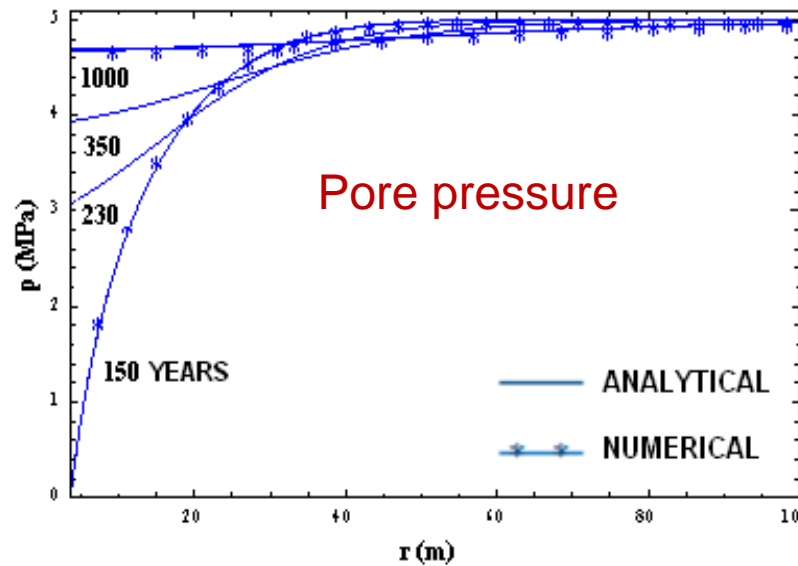
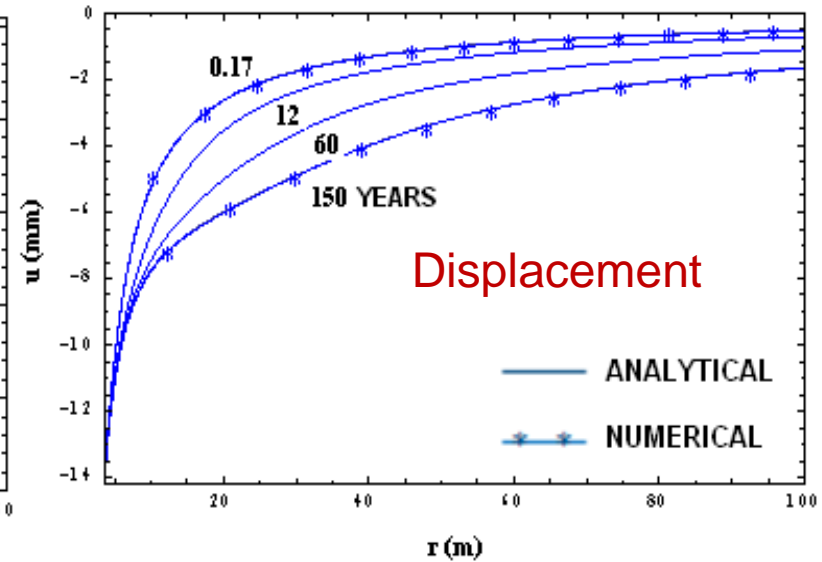
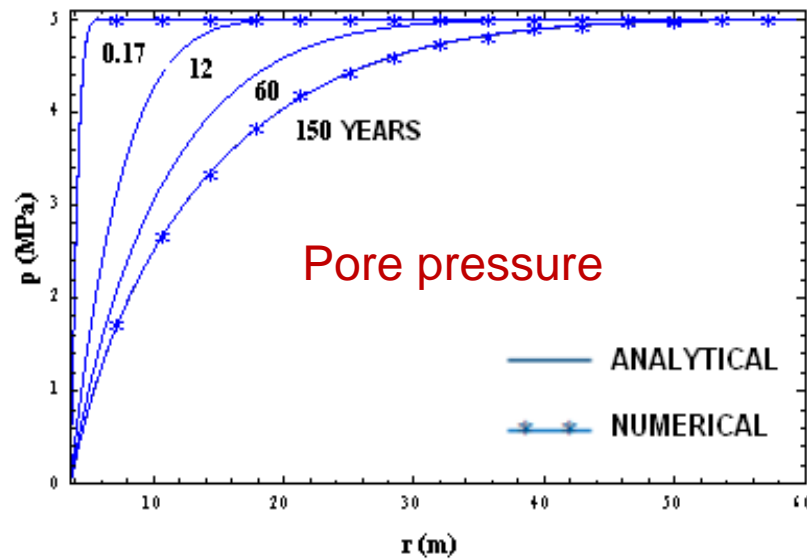
$$d\sigma_{ij} = D_{ijkl}^T d\varepsilon_{kl}$$

$$d\sigma_{ij} = D_{ijkl}^E d\varepsilon_{kl} - bp\delta_{ij}$$

~ 0

$$d\sigma_{ij} = D_{ijkl}^T d\varepsilon_{kl} - bp\delta_{ij}$$

Comparisons : analytical & numerical Poroelasticity



Poro-viscoplastic damageable (iso) behavior law

- ④ Strain partition

$$\dot{\boldsymbol{\varepsilon}} = \underbrace{\dot{\boldsymbol{\varepsilon}}^e}_{\text{elastic}} + \underbrace{\dot{\boldsymbol{\varepsilon}}^{vp}}_{\text{viscoplastic}}$$


- ④ Damageable viscoplastic Lemaitre's law

$$\dot{\boldsymbol{\varepsilon}}^{vp} = \frac{3}{2} \dot{\gamma} \frac{\mathbf{s}}{\sigma_{eq}} \quad \text{with} \quad \dot{\gamma} = \sqrt{\frac{2}{3} \dot{\boldsymbol{\varepsilon}}^{vp} : \dot{\boldsymbol{\varepsilon}}^{vp}}$$

$$\sigma_{eq} = \sqrt{\frac{3}{2} \mathbf{s} : \mathbf{s}} \quad \dot{\gamma} = \frac{\dot{r}}{1-D} \quad \dot{r} = \left\langle \frac{\sigma_{eq}}{(1-D)K r^{1/M}} \right\rangle^N$$

$$\dot{D} = \left\langle \frac{\sigma_{eq}}{A} \right\rangle^R (1-D)^{-k}$$

- ④ + superposition \rightarrow Damageable Poro-viscoplastic model



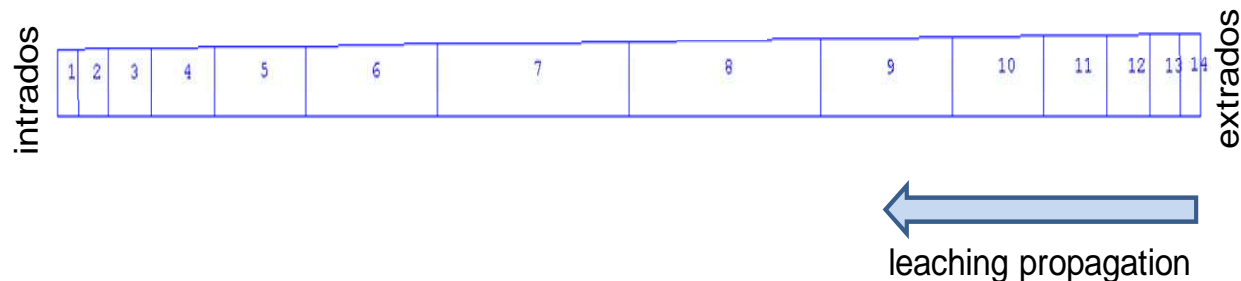
$$\sigma'_{ij} = \sigma_{ij} + bp\delta_{ij}$$

Parametric study relative to different kinetics of Lining support degradation

Normal & Accidental Scenarios

- ④ Undamaged poroplastic liner
- ④ Brittle poroplastic liner
- ④ **Progressive leaching of the liner from outer to inner face**

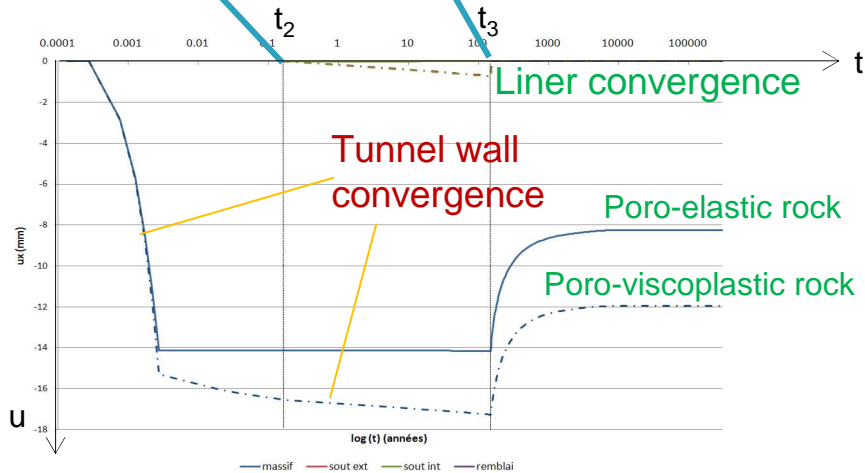
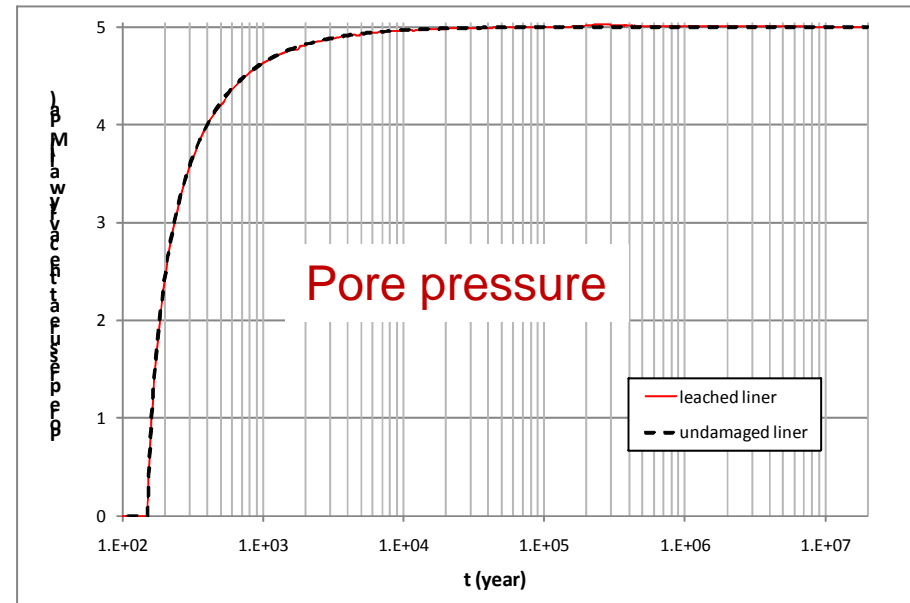
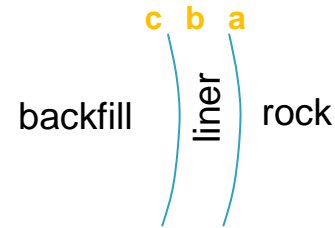
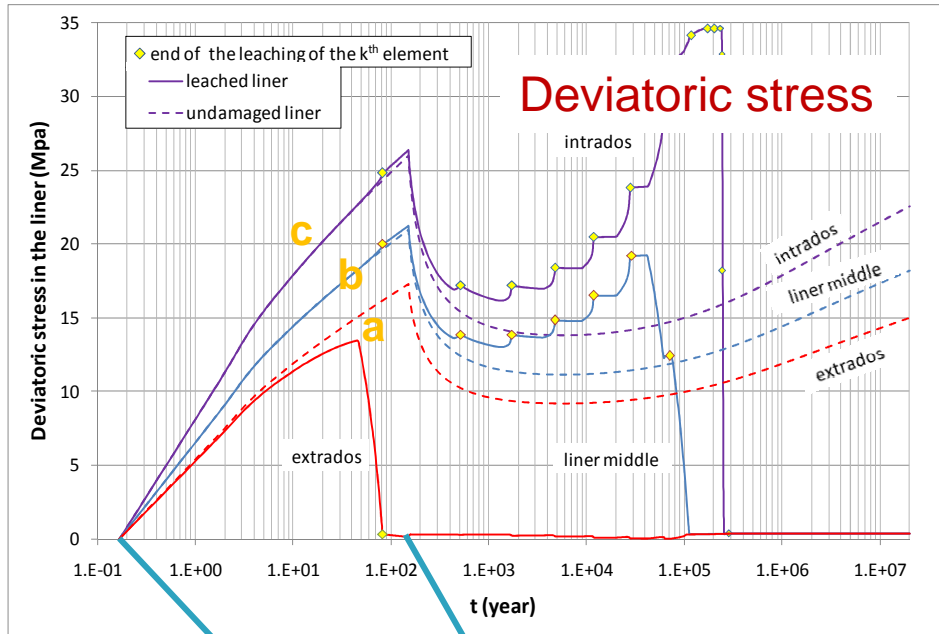
Simplified model, $e_d(t) = e_d(\tau^0) \sqrt{\frac{t}{\tau^0}}$ focus on rock mass behaviour
(Torrenti & al., 2008)



After leaching front: elastic moduli and strength $\rightarrow \sim 0$

Lining support degradation

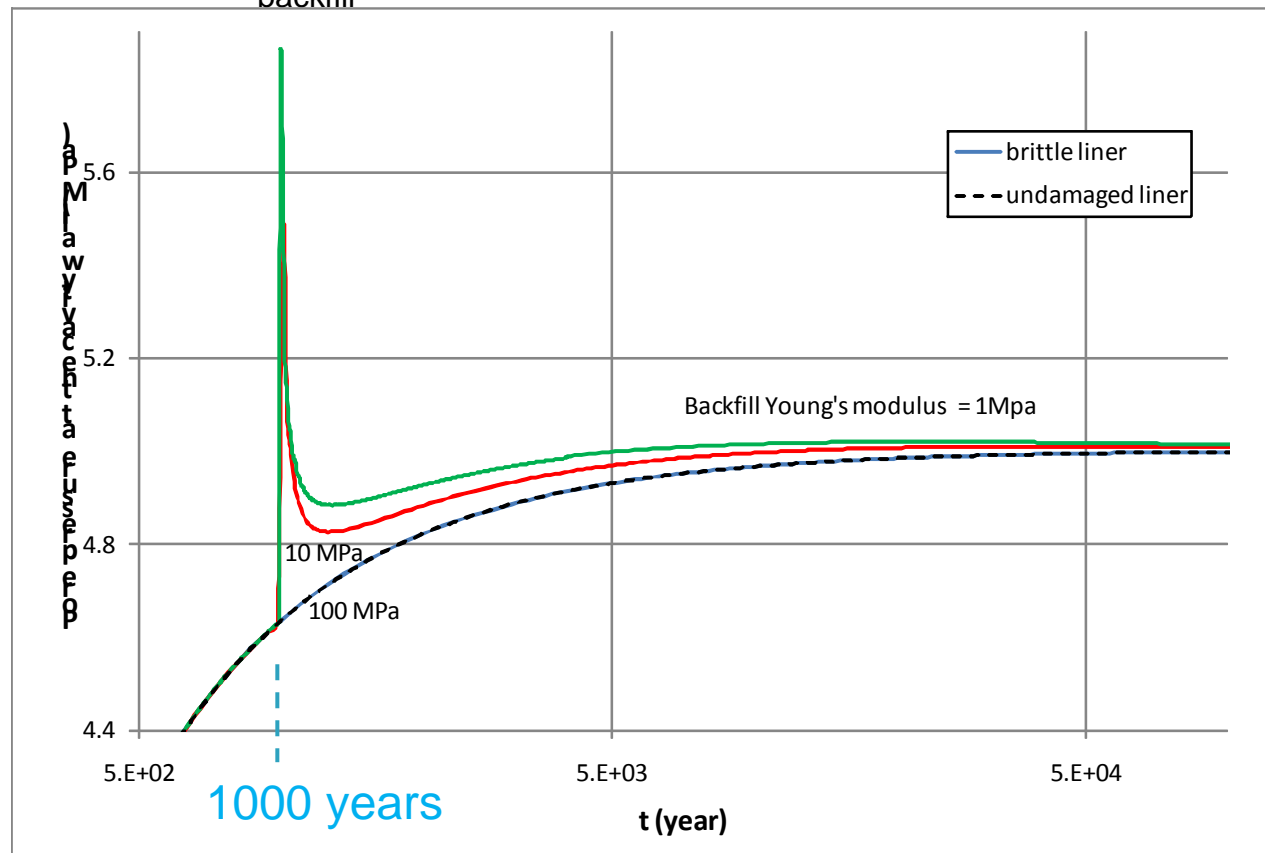
Progressive leaching front : extrados → intrados



Backfill compaction

Brittle poroplastic liner : sudden failure at $t = 1000$ years

- $E_{\text{backfill}} = 100 \text{ MPa}$
- $E_{\text{backfill}} = 10 \text{ MPa}$
- $E_{\text{backfill}} = 1 \text{ MPa}$



Poro-viscoplastic behavior + anisotropic damage

$$\begin{aligned}\dot{\boldsymbol{\sigma}} &= \mathbb{C}^\sigma(\mathbf{D}) : \dot{\boldsymbol{\varepsilon}} - \dot{p}_w \mathbf{B}(\mathbf{D}) - \mathbb{C}(\mathbf{D}) : \dot{\boldsymbol{\varepsilon}}^{vp} \\ \dot{\phi} &= \mathbf{B}^\phi(\mathbf{D}) : \dot{\boldsymbol{\varepsilon}} + \beta(\mathbf{D}) \dot{p}_w\end{aligned}$$

$\mathbf{D} = 2^{\text{nd}}$ order tensor

$$\begin{aligned}f_d &= f_d(\boldsymbol{\varepsilon}^+; \mathbf{D}) = \sqrt{\boldsymbol{\varepsilon}^+ : \boldsymbol{\varepsilon}^+} - (r_0 + r_1 \text{tr}(\mathbf{D})) \\ \dot{\mathbf{D}} &= \left(\frac{\boldsymbol{\varepsilon}^+ \otimes \boldsymbol{\varepsilon}^+}{r_1 \text{tr}(\boldsymbol{\varepsilon}^+) \|\boldsymbol{\varepsilon}^+\|} \right) : \dot{\boldsymbol{\varepsilon}}^+\end{aligned}$$

$$\boldsymbol{\varepsilon}^+ = \sum_{K=1}^3 H(\varepsilon_K) \varepsilon_K \mathbf{e}_K \otimes \mathbf{e}_K = \mathbb{P}^+ : \boldsymbol{\varepsilon}$$

$$\dot{\boldsymbol{\varepsilon}}^{vp} = \sqrt{\frac{3}{2}} \dot{\gamma}_{vp} \frac{\frac{3}{2} \frac{\tilde{\mathbf{s}}'}{q'} + \frac{\alpha(\boldsymbol{\delta} - \mathbf{D})}{3}}{\left\| \frac{3}{2} \frac{\tilde{\mathbf{s}}'}{q'} + \frac{\alpha(\boldsymbol{\delta} - \mathbf{D})}{3} \right\|}$$

$$\tilde{\boldsymbol{\sigma}}' = \mathbb{M}(\mathbf{D}) : \boldsymbol{\sigma}' = \mathbb{M}(\mathbf{D}) : (\boldsymbol{\sigma} + p_w \mathbf{B}(\mathbf{D}))$$

$$\dot{\gamma}_{vp} = \sqrt{\frac{2}{3}} \left\langle \frac{\tilde{q}' + \alpha \tilde{\boldsymbol{\sigma}}' - k}{K \gamma_{vp}^{\frac{1}{M}}} \right\rangle_N \left\| \frac{3}{2} \frac{\tilde{\mathbf{s}}'}{q'} + \frac{\alpha(\boldsymbol{\delta} - \mathbf{D})}{3} \right\|$$

→ 1D FEM code in C++ (Pereira, 2005)

Conclusions

- Multi-physical phenomena (HM-diffusion, creep, damage, leaching).
Structural response depends on ratio between characteristic times
- Progressive (slow) leaching of liner
Well compacted backfill (brutal liner failure) } → no excess pore pressure
- Interesting result : complex transitional behavior at rock/liner & liner/backfill interfaces during hydraulic readjustment (possible theoretical gap-opening leading to hydraulic conductivity increase...)
- Perspective: improvements by accounting for:
 - anisotropic damage (on-going)
 - desaturation/resaturation phases