

Hydro-mechanical analysis of the fracturing induced by the excavation of nuclear waste repository galleries using shear banding

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Long-term management of radioactive wastes



Intermediate (long-lived) & high activity wastes



Deep geological disposal Repository in deep geological media with good confining properties (Low permeability

K<10⁻¹² m/s)

Underground structures

= network of galleries



Disposal facility of Cigéo project in France (Labalette et al., 2013)

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Fracture modellin

Anisotrop

Repository phases



Type C wastes (Andra, 2005)

Excavation Damaged Zone (EDZ)





Fracturing & permeability increase (several orders of magnitude)

Opalinus clay in Switzerland (Bossart et al., 2002)



Callovo-Oxfordian claystone (COx)

Sedimentary clay rock (France).





Borehole core samples (Andra, 2005)

- Underground research laboratory

Feasibility of a safe repository France (Meuse / Haute-Marne, Bure)



Context

Fracture modellii

Anisotrop

Nater transfe

Context 1.

- Fracturing



- Hydraulic transfer

Large-scale ventilation experiment (SDZ).

Characterise the effect of gallery ventilation on the hydraulic transfer.





Issues: Drainage / desaturation reproduction.

Objectives

Better understand, predict, and model the behaviour of the EDZ in partially saturated clay rock, at repository scale.



Fracture description

EDZ development.

Constitutive models

Mechanics: Anisotropic behaviour. Influence on fracturing.

Coupled: **Hydro-mechanical coupling**. Fracture - permeability evolution.

Numerical modelling

Finite element method.

Reproduction of **fractures and EDZ** in rock.

Influence of **fracturing** on **water transfer** around galleries.

Context

- 2. Fracture modelling with shear bands
- 3. Influence of mechanical anisotropy
- 4. Permeability evolution and water transfer
- 5. Conclusions

2.1. Material rupture



Axial shortening, ΔH [mm]

- Fracture modelling

Shear bands are observed in many geomaterials.

COx: 75% of fractures in mode II (shear).



 σ_1

Shear failure

- Mechanisms of rock mass failure around gallery



(after Diederichs, 2003)

Shear strain localisation (continuous approach)

2.2. Shear strain localisation

2.2.1. Examples

Brutal concentration of strain in narrow zones.



Sand (Alshibli et al., 2003)



Sandstones, Muddy Mountains, Nevada (H. Fossen, 2010)



Ductile shear in quartz and pyroxene (Christopher et al., 2006)

Water transfe

2.2.2. Finite element methods

- Classical FE

Mesh dependency

Need to introduce an **<u>internal length</u>** scale for a correct modelling of the post-peak behaviour.



- Regularisation

Enrichment of the kinematics :

The continuum is enriched with microstructure effects. Macro-kinematics + micro-kinematics

Macro Ω:

Micro Ω^m:

$$F_{ij} = \frac{\partial u_i}{\partial x_j} = \varepsilon_{ij} + r_{ij}$$

$$\upsilon_{ij} = \frac{\partial u_i^m}{\partial x_j} = \mathcal{E}_{ij}^m + r_{ij}^m$$



2.2.3. Coupled local second gradient model

- Balance equations

Additional terms related to microstructure effects.

Porous media, biphasic : solid + fluid (Collin et al., 2006)

Unsaturated condition, compressibility of the solid grains, anisotropy, k_w variation (Pardoen et al., 2015)

$$S_{r,w}$$
 $b = 1 - \frac{K}{K_s}$ $\sigma_{ij} = \sigma'_{ij} + b S_{r,w} p_w \delta_{ij}$

- Second gradient mechanical law

The double stress Σ_{ijk} requires a constitutive law. Linear elastic law function of the (micro) second gradient of displacement field.



Internal length scale

D represents the physical microstructure





Solid grainsWaterAir





Anisotrop

2.3. Constitutive models for COx

- Mechanical law - 1st gradient model

Isotropic elasto-plastic internal friction model Non-associated plasticity, Van Eeckelen yield surface :

$$F \equiv II_{\hat{\sigma}} - m\left(I_{\sigma'} + \frac{3c}{\tan\varphi_c}\right) = 0$$

φ hardening / c softening

$$c = c_0 + \frac{\left(c_f - c_0\right)\hat{\varepsilon}_{eq}^p}{B_c + \hat{\varepsilon}_{eq}^p} \longrightarrow \text{Strain localisation}$$

F=0 φc,f φc,0 Γ Γ Γ Γ Γ Γ Γ Γ Γ

zone

Πô

zone

- Hydraulic law

Fluid mass flow (advection, Darcy) :

$$f_{w,i} = -\rho_w \frac{k_{w,ij} k_{r,w}}{\mu_w} \left(\frac{\partial p_w}{\partial x_j} + \rho_w g_j \right)$$

Water retention and permeability curves (Mualem - Van Genuchten's model)

2.4. Gallery excavation modelling



HM modelling in 2D plane strain state

Gallery in COx // σ_h

Excavation: σ_r , p_w at gallery wall $\rightarrow 0$



Effect of stress anisotropy

- Localisation zone



→ For an isotropic mechanical behaviour, the appearance and shape of the strain localisation are mainly due to mechanical effects linked to the anisotropic stress state.

- Convergence:

Important during the excavation Anisotropic convergence Experimental results (GED - Andra's URL) No strain localisation





2.5. Conclusions and outlooks

- Conclusions

- \checkmark Reproduction of EDZ with shear bands.
- ✓ Shape and extent of EDZ governed by anisotropic stress state.



- Next steps ...

- X Mechanical rock behaviour.
 - \rightarrow Material anisotropy, gallery // $\sigma_{\rm H}$.
- X HM coupling in EDZ.
 - \rightarrow Influence of fracturing on hydraulic properties.
- X Gallery air ventilation and water transfer (drainage / desaturation).



2. Fracture modelling with shear bands

3. Influence of mechanical anisotropy

- 4. Permeability evolution and water transfer
- 5. Conclusions

3.1. Anisotropic mechanical behaviour

Sedimentary rocks with bedding planes



 e_2

Stratigraphy related to microstructure of Opalinus clay, Switzerland (Wenk et al. 2008).



→ Cross-anisotropy



Behaviour depends on:

- the microstructure
- the loading direction

Contex

Fracture modelli

Anisotropy

Water transfei



- Linear elasticity :

Cross-anisotropic (5 param.) + Biot's coefficients

 $E_{\scriptscriptstyle //}, E_{\scriptscriptstyle \perp},
u_{\scriptscriptstyle ///},
u_{\scriptscriptstyle //\perp}, G_{\scriptscriptstyle //\perp} = b_{\scriptscriptstyle //}, b_{\scriptscriptstyle \perp}$

- Plasticity :

Cohesion anisotropy with fabric tensor

$$c_0 = a_{ij} l_i l_j$$
 $l_i = \sqrt{\frac{\sigma_{i1}^{'2} + \sigma_{i2}^{'2} + \sigma_{i3}^{'2}}{\sigma_{ij}^{'}\sigma_{ij}^{'}}}$

Cross-anisotropy

$$c_{0} = \overline{c} \left(1 + A_{////} (1 - 3l_{2}^{2}) + b_{1}A_{////}^{2} (1 - 3l_{2}^{2})^{2} + ... \right)$$

$$e_{2}$$

$$c_{1}$$

$$c_{2}$$

$$c_{2}$$

$$c_{3}$$

$$c_$$

3.2. Gallery excavation modelling for isotropic initial stress state



Context

Anisotropy

ater transfer

Conclusion

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	Fracture modelling	Anisotropy			2
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3.3. Conclusions and outlooks

- Conclusions

- ✓ Reproduction of EDZ in both directions.
- ✓ Shape and extent of EDZ governed by:
 - anisotropic stress state.
 - anisotropic mechanical behaviour.
- ✓ Long-term convergence with viscosity.

- Next steps ...

- X HM coupling in EDZ.
 - \rightarrow Influence of fracturing on hydraulic properties.
- X Gallery air ventilation and water transfer.





Anisotropy

Nater transfer

- 1. Context
- 2. Fracture modelling with shear bands
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4.1. Large-scale experiment of gallery ventilation (SDZ)

Characterise the effect of gallery ventilation on the hydraulic transfer around it.

- → drainage / desaturation
- \rightarrow exchange at gallery wall







4.2. Permeability variation in fractured zone

HM coupling in the EDZ.

4.2.1. Saturated permeability in boreholes







Fracture and rock matrix permeabilities

- \rightarrow Capture k_w evolution
- \rightarrow Relation to fractures

4.2.2. Evolution of intrinsic water permeability

Various approaches: deformation, damage, cracks...

- Fracture permeability

Cubic law for parallel-plate approach (Witherspoon 1980; Snow 1969, Olivella and Alonso 2008)



$$k_{w} = \frac{b^{3}}{12B}$$

$$b = b_{0} + B \left\langle \varepsilon^{n} - \varepsilon^{n}_{0} \right\rangle$$

$$k_{w} = k_{w,0} \left(1 + A \left\langle \varepsilon^{n} - \varepsilon^{n}_{0} \right\rangle \right)^{3}$$

Localised deformation Fracture initiation

- Empirical law

Related to strain localisation effect

Permeability variation threshold

$$k_{w,ij} = k_{w,ij,0} \left(1 + \beta_{per} \left\langle YI - YI^{thr} \right\rangle \hat{\varepsilon}_{eq}^{3} \right) \qquad \qquad YI = \frac{\Pi_{\hat{\sigma}}}{\Pi_{\hat{\sigma}}^{p}}$$

4.3. Hydraulic boundary condition for exchanges at gallery wall

- Classical approach Instantaneous equilibrium (Kelvin's law) R

$$RH = \frac{\rho_v}{\rho_{v,0}} = exp\left(\frac{-p_c M_v}{\rho_w RT}\right)$$

- Experimental

Drainage / desaturation \rightarrow Progressive hydraulic transfer & equilibrium

- Non-classical mixed boundary condition
Liquid water + water vapour
$$\overline{q}_w = \overline{S} + \overline{E}$$

- Seepage flow :

$$\begin{cases} \overline{S} = K^{pen} (p_w^{\Gamma} - p_{atm})^2 & \text{if } p_w^{\Gamma} \ge p_w^{air} \text{ and } p_w^{\Gamma} \ge p_{atm} \\ \overline{S} = 0 & otherwise \end{cases}$$

otherwise

- Evaporation flow : (Nasrallah and Perre, 1988)

$$\overline{E} = \alpha_{v} \left(\rho_{v}^{\Gamma} - \rho_{v}^{air} \right)$$



Water transfer

4.4. Modelling of excavation and ventilation experiment (SDZ)





- Desaturation EDZ / w reproduction

Horizontal boreholes

At gallery wall



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5. Conclusions

Conclusions

Better understand, predict, and model the behaviour of the EDZ in partially saturated clay rock, at repository scale.





Fracture description

Strain localisation. EDZ with shear bands.

Constitutive models

Mechanics: anisotropy, viscosity.

Coupled: Shear bands / permeability.

Numerical modelling

EDZ for both gallery directions.

Shape and extent related to anisotropies.

Influence of shear bands HM aspects.

Water transfer.

Contribution : Provide new elements for the prediction and understanding of the HM behaviour of the EDZ.

Innovations : Fracturing process is predicted on a **repository scale** with **shear bands**. Strain localisation effects are taken into account in **coupled processes** (water flow).

	Fracture modelling	Anisotropy		Conclusion
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5. Conclusions

Acknowledgments:







